

MiTek, Inc. 16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200

Re: 24100211-A Thyret-Roof

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Carter Components (Newton Falls, OH).

Pages or sheets covered by this seal: I68859347 thru I68859348

My license renewal date for the state of Ohio is December 31, 2025.



October 15,2024

Tony Miller

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design per ANSI/TPI 1, Chapter 2.

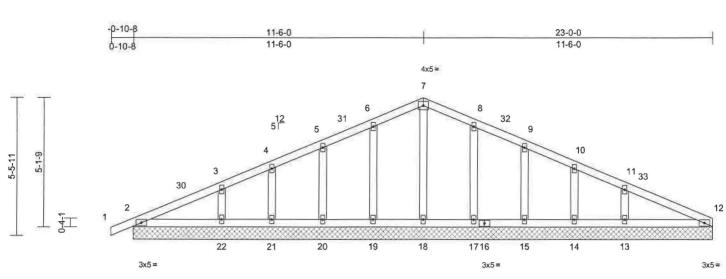
Job	Truss	Truss Type	Qty	Ply	Thyret-Roof	
24100211-A	A1GE	COMMON	1	1	Job Reference (optional)	168859347

Carter Components (Newton Falls), Newton Falls, OH - 44444,

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Fri Oct 11 12:38:41 ID:hpnpk5C392HmifuzmA6OuxyUUS0-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

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23-0-0

Scale = 1:43.9

Loading TCLL (roof) Snow (Ps/Pg) TCDL BCLL BCDL	(psf) 30,0 23,1/30,0 10,0 0,0 10,0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018/TPI201	CSI TC BC WB 4 Matrix-MS	0.13 0.13 0.06	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 12	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 84 lb	GRIP 197/144 FT = 20%
	6-0-0 oc purlins. Rigid ceiling directly bracing. (size) 2=23-0-0, 14=23-0-0 21=23-0-0 21=23-0-0 21=23-0-0 21=23-0-0 21=23-0-0 21=23-0-0 21=23-0-0 14=-13 (L 17=-20 (L 20=-20 (L 22=-32 (L Max Grav 2=232 (L0 13=356 (L 15=253 (L 18=173 (L 20=252 (L)	18), 23=68 (LC 18) 10), 13=-35 (LC 15), C 15), 15=-21 (LC 15 C 15), 19=-21 (LC 14 C 14), 21=-14 (LC 10 C 2), 12=150 (LC 2), C 37), 14=153 (LC 2) C 2), 17=249 (LC 2) C 2), 19=250 (LC 2) C 2), 21=159 (LC 2) C 2), 21=159 (LC 2) C 36), 23=232 (LC 2)	I or this de 2) Wind: Vasd= Ke=1.(0, exterio 0-0, Interiou 0-0, Interiou 0-0, 23-0-0 vertica forces DOL=1), 3) Truss (0, only. F see St or coms 4) TCLL: 2), Plate D 2), Partial 1), 5) Roofd slope.	7-18=-132/0, 6-1 4-21=-141/38, 3- 9-15=-207/45, 10 ncced roof live loads h sign. SCE 7-16; Vult=115r 91mph; TCDL=6.0psf; 0; Cat. II; Exp B; Encl 200 and C-C Exteri (1) 2-1-8 to 8-6-0, Ex (1) 14-6-0 to 20-0-0, zone; cantilever left al left and right exposed % MWFRS for reaction 60 plate grip DOL=1. lesigned for wind load or studs exposed to w indard Industry Gable ult qualified building of ASCE 7-16; Pr=30.0 p OL=1.15); Pg=30.0 p OL=1.15; Pg=30.0 p J: 5 Plate DOL=1.15; y Exp.; Ce=1.0; Cs=1. sign snow loads have	22=-241/5)-14=-138 ave been mph (3-see BCDL=6. osed; MW or(2E)-0- terior(2R) Exterior(2R) Exterior(2R) Exterior(2C) Exter	58, 8-17=-211 (38, 11-13=-2 considered for opsf; h=25ft; FRS (envelo 10-8 to 2-1-8, 8-6-0 to 14-6 E) 20-0 to posed ; end Lumber ane of the tru al to the face ils as applica s per ANSI/TI 1 psf (Lum Dugh Cat B; 10 eed to accour	//49, 249/59 or -0, -0, sss), ble, Pl 1. 1.15 t for	bea 21 I join Ib u join 14) This Inte	ring plat b uplift at t 21, 32 plift at jot t 13 and s truss is mationa 02.10.2 a CASE(S	te capa at joint Ib uplif int 15, 4 Ib up a design Il Resid and ref) Star	able of withstandi 19, 20 lb uplift at t at joint 22, 20 ll 13 lb uplift at joi plift at joint 2, ned in accordann dential Code sect erenced standar ndard	tions R502.11.1 and
FORCES TOP CHORD BOT CHORD	(lb) - Maximum Com Tension 1-2=0/38, 2-3=-73/52 4-5=-50/61, 5-6=-60/ 7-8=-69/92, 8-9=-60/ 10-11=-59/20, 11-12 2-22=-25/51, 21-22= 19-20=-19/51, 18-19 15-17=-19/51, 14-15 12-13=-19/64	pression/Maximum 2, 3-4=-65/46, 76, 6-7=-69/92, 64, 9-10=-50/40, =-75/43 =19/51, 20-21=-19/51 =-19/51, 17-18=-19/5	load of overha 8) All plat 9) Gable 10) Gable 1, 11) This tru 1 chord I	ss has been designed 12.0 psf or 2.00 times 195 non-concurrent wi es are 2x4 MT20 unle equires continuous bo tuds spaced at 2-0-0 ss has been designed ve load nonconcurren ings are assumed to f	a flat roof le ith other liness otherwin ottom chor oc. I for a 10.0 t with any	bad of 23.1 p ve loads. se indicated. d bearing. D psf bottom other live loa	sfon		,	"HILLING OF PENNS	TON MILE PESA PEGIST	ERED NOT

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oblapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, arection and bracing of trusses and truss systems, see ANSI/TFH Quality Critoria, and DSE-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

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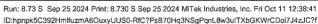
October 15,2024

Job	Truss	Truss Type	Qty	Ply	Thyret-Roof	
24100211-A	A1	COMMON	6	1	Job Reference (optional)	168859348

Carter Components (Newton Falls), Newton Falls, OH - 44444,

-D-10-8

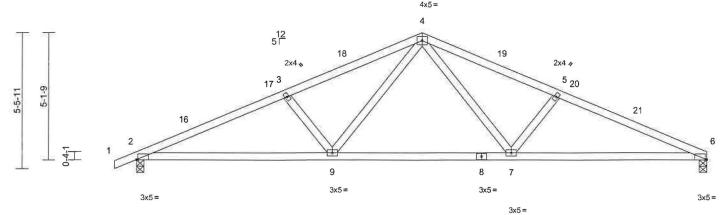
0-10-8



Page: 1

 6-0-14
 11-6-0
 16-11-2
 23-0-0

 6-0-14
 5-5-2
 5-5-2
 6-0-14



1	7-10-10	15-1-6	23-0-0	E.
	7-10-10	7-2-13	7-10-10	

Scale = 1:44.6

Plate Offsets (X, Y):	[2:0-0-10,Edge],	[6:0-0-10,Edge]
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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	în	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	30.0	Plate Grip DOL	1.15	TC	0.54	Vert(LL)	-0.13	7-15	>999	360	MT20	197/144
Snow (Ps/Pg)	23.1/30.0	Lumber DOL	1.15	BC	0.77	Vert(CT)	-0.26	7-15	>999	240		
TCDL	10.0	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.06	6	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-MS								
BCDL	10.0										Weight: 76 lb	FT = 20%

Unbalanced snow loads have been considered for this

This truss has been designed for greater of min roof live

load of 12.0 psf or 2.00 times flat roof load of 23.1 psf on

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2 BRACING TOP CHORD Structural wood sheathing directly applied or 3-3-5 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. **REACTIONS** (size) 2=0-3-8, 6=0-3-8 Max Horiz 2=68 (LC 14) Max Uplift 2=-23 (LC 14), 6=-12 (LC 15) Max Grav 2=1221 (LC 2), 6=1149 (LC 2) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/38, 2-3=-2312/123, 3-4=-2043/123, 4-5=-2049/127, 5-6=-2318/127 BOT CHORD 2-9=-64/2090, 7-9=-7/1383, 6-7=-69/2098 3-9=-526/122, 4-9=-14/726, 4-7=-16/733, WEBS

5-7=-530/123

DOL=1.60 plate grip DOL=1.60

Unbalanced roof live loads have been considered for

Wind: ASCE 7-16; Vult=115mph (3-second gust)

Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Ke=1.00; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 8-6-0, Exterior(2R) 8-6-0 to 14-6-0, Interior (1) 14-6-0 to 20-0-0, Exterior(2E) 20-0-0 to 23-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber

NOTES 1) Unb

2)

3)

this design.

applied or 10-0-0 oc
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
a) Beo-3-8
a) All bearings are assumed to be SPF No.2.
b) Brouide mechanical concentring for the true to the second sec

5)

6)

slope.

design.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 2 and 12 lb uplift at joint 6.
 This truss is designed in accordance with the 2018

overhangs non-concurrent with other live loads.

International Residential Code sections R502,11.1 and R802.10.2 and referenced standard ANSI/TPI 1. LOAD CASE(S) Standard

> TONY R. MILLER PES47 1 October 15,2024

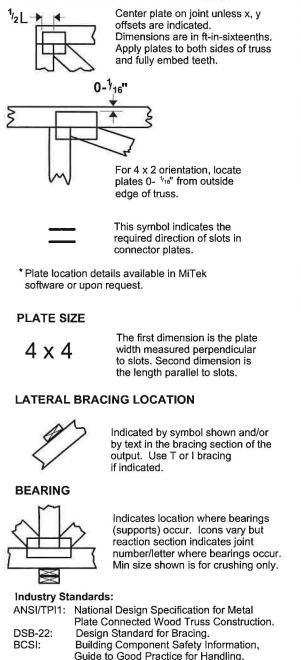


TCLL: ASCE 7-16; Pr=30.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=30.0 psf; Ps=23.1 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

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Symbols

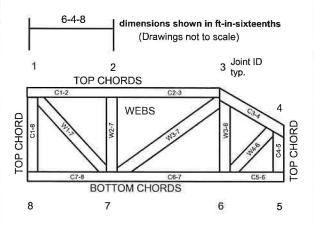
PLATE LOCATION AND ORIENTATION



Installing, Restraining & Bracing of Metal

Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282 ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- 3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- 7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- 8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer,
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- The design does not take into account any dynamic or other loads other than those expressly stated.

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